

Plane Stress

Displacement function: u & v in-plane displacement

$$u = \sum_i \sum_j a_{ij} \xi^i \eta^j$$

$$v = \sum_i \sum_j b_{ij} \xi^i \eta^j$$

Strain Energy Calculation: ✓

$$U = \frac{Eh}{2(1-\nu^2)} \int_{-1}^1 \int_{-1}^1 \left[\left(\frac{\partial u}{\partial x} \right)^2 + \left(\frac{\partial v}{\partial y} \right)^2 + 2\nu \frac{\partial u}{\partial x} \frac{\partial v}{\partial y} + \frac{(1-\nu)}{2} \left(\frac{\partial u}{\partial y} \right)^2 + (1-\nu) \frac{\partial u}{\partial y} \frac{\partial v}{\partial x} + \frac{(1-\nu)}{2} \left(\frac{\partial v}{\partial x} \right)^2 \right] |J| d\xi dy$$

$$|J| = \frac{\partial x}{\partial \xi} \cdot \frac{\partial y}{\partial \eta} - \frac{\partial x}{\partial \eta} \cdot \frac{\partial y}{\partial \xi}$$

$$\frac{\partial u}{\partial x} = \frac{\partial u}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial u}{\partial \eta} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial u}{\partial y} = \frac{\partial u}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial u}{\partial \eta} \frac{\partial \eta}{\partial y}$$

$$\frac{\partial v}{\partial x} = \frac{\partial v}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial v}{\partial \eta} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial v}{\partial y} = \frac{\partial v}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial v}{\partial \eta} \frac{\partial \eta}{\partial y}$$

$$\frac{\partial u}{\partial \xi} = \sum_{i=1}^{\infty} \sum_{j=0}^{\infty} a_{ij} \xi^{i-1} \eta^j$$

$$\frac{\partial u}{\partial \eta} = \sum_{i=0}^{\infty} \sum_{j=1}^{\infty} a_{ij} \xi^i \eta^{j-1}$$

$$\frac{\partial v}{\partial \xi} = \sum_{i=1}^{\infty} \sum_{j=0}^{\infty} b_{ij} \xi^{i-1} \eta^j$$

$$\frac{\partial v}{\partial \eta} = \sum_{i=0}^{\infty} \sum_{j=1}^{\infty} b_{ij} \xi^i \eta^{j-1}$$

Side Note:

Polynomial series: (multi-dimensional)

$$u(x,y) = \sum_{i=0}^{\infty} \sum_{j=0}^{\infty} a_{ij} x^i y^j$$

$$= a_{00} + a_{01} y^1 + a_{02} y^2 + a_{03} y^3 + a_{04} y^4 + \dots$$

$$+ a_{10} x y + a_{11} x y^2 + a_{12} x y^3 + a_{13} x y^4 + \dots$$

$$+ a_{20} x^2 y + a_{21} x^2 y^2 + a_{22} x^2 y^3 + a_{23} x^2 y^4 + \dots$$

$$+ \dots$$

$$\frac{\partial u}{\partial x} = a_{11} y + a_{12} y^2 + a_{13} y^3 + a_{14} y^4 + \dots$$

$$+ a_{21} x y + a_{22} x y^2 + a_{23} x y^3 + a_{24} x y^4 + \dots$$

$$+ \dots$$

$$\Rightarrow \frac{\partial u}{\partial x} = \sum_{i=1}^{\infty} \sum_{j=0}^{\infty} a_{ij} x^{i-1} y^j$$

$$\Rightarrow \frac{\partial u}{\partial y} = \sum_{i=0}^{\infty} \sum_{j=1}^{\infty} a_{ij} x^i y^{j-1}$$

To solve $\frac{\partial \sigma}{\partial x}$, $\frac{\partial \sigma}{\partial y}$, $\frac{\partial \eta}{\partial x}$ and $\frac{\partial \eta}{\partial y}$

$$\sigma = C_1 \sigma^2 \eta + C_2 \sigma^2 + C_3 \sigma + C_4 \sigma \eta^2 + C_5 \eta^2 + C_6 \eta + C_7 \sigma \eta + C_8$$

$$\eta = D_1 \sigma^2 \eta + D_2 \sigma^2 + D_3 \sigma + D_4 \sigma \eta^2 + D_5 \eta^2 + D_6 \eta + D_7 \sigma \eta + D_8$$

Using Implicit Differentiation :

take $\frac{\partial}{\partial x}$ on both sides , consider $\sigma(x,y)$, $\eta(x,y)$

$$\left. \begin{array}{l} 1 = C_1 \cdot 2 \sigma \frac{\partial \sigma}{\partial x} \eta + C_1 \sigma^2 \frac{\partial \eta}{\partial x} + C_2 \cdot 2 \sigma \frac{\partial \sigma}{\partial x} + C_3 \frac{\partial \sigma}{\partial x} \\ \quad + C_4 \frac{\partial \sigma}{\partial x} \eta^2 + C_4 \sigma \cdot 2 \eta \frac{\partial \eta}{\partial x} + C_5 \cdot 2 \eta \frac{\partial \eta}{\partial x} + C_6 \frac{\partial \eta}{\partial x} + C_7 \sigma \frac{\partial \eta}{\partial x} \\ 0 = D_1 \cdot 2 \sigma \frac{\partial \sigma}{\partial x} \eta + D_1 \sigma^2 \frac{\partial \eta}{\partial x} + D_2 \cdot 2 \sigma \frac{\partial \sigma}{\partial x} + D_3 \frac{\partial \sigma}{\partial x} \\ \quad + D_4 \frac{\partial \sigma}{\partial x} \eta^2 + D_4 \sigma \cdot 2 \eta \frac{\partial \eta}{\partial x} + D_5 \cdot 2 \eta \frac{\partial \eta}{\partial x} + D_6 \frac{\partial \eta}{\partial x} + D_7 \sigma \frac{\partial \eta}{\partial x} \end{array} \right\}$$

2 equations , 2 variables $\frac{\partial \sigma}{\partial x}$ & $\frac{\partial \eta}{\partial x}$, solvable

Same for $\frac{\partial \eta}{\partial y}$ and $\frac{\partial \eta}{\partial y}$, solvable

$$\Rightarrow \frac{\partial \sigma}{\partial x}(x,y) , \frac{\partial \sigma}{\partial y}(x,y) , \frac{\partial \eta}{\partial x}(x,y) \text{ and } \frac{\partial \eta}{\partial y}(x,y)$$

they are all just functions of x and y

$$\Rightarrow \frac{\partial^2 \sigma}{\partial x^2} , \frac{\partial^2 \sigma}{\partial y^2} , \frac{\partial^2 \eta}{\partial x^2} , \frac{\partial^2 \eta}{\partial y^2} , \frac{\partial^2 \sigma}{\partial x \partial y} \text{ and } \frac{\partial^2 \eta}{\partial x \partial y}$$

these will be used on calculating buckling strain energy

Work of Applied Load :

$$T = \int_S \frac{1}{|t|} \left[(-f_n t_y + f_t t_x) u + (f_n t_x + f_t t_y) v \right] \cdot \left[(t_x \frac{\partial x}{\partial s} + t_y \frac{\partial y}{\partial s}) ds + (t_x \frac{\partial x}{\partial \eta} + t_y \frac{\partial y}{\partial \eta}) d\eta \right]$$

↑
on one edge

$$T = \sum_{i=1}^4 T_i \quad ?$$

For this example need mappings for edges
 — For side $S=1$, $f_n = 0h$, $f_t = 0$, $t_x = t_y = 1$, $|t| = \sqrt{2}$

$$\begin{aligned} T &= \int_S \frac{1}{|t|} \left[(-f_n t_y + f_t t_x) u + (f_n t_x + f_t t_y) v \right] \cdot \left[(t_x \frac{\partial x}{\partial s} + t_y \frac{\partial y}{\partial s}) ds + (t_x \frac{\partial x}{\partial \eta} + t_y \frac{\partial y}{\partial \eta}) d\eta \right] \\ &= \int_S \frac{1}{\sqrt{2}} \left[-0h \cdot (1) \cdot u + 0h \cdot (1) \cdot v \right] \cdot \left[(1) \cdot 4 + (1) \cdot 0 \right] ds \\ &\quad + \left[(1) \cdot 2\sqrt{2} + (1) \cdot 2\sqrt{2} \right] d\eta \\ &= \int_{-1}^1 \frac{h}{\sqrt{2}} (-0 \cdot u + 0 \cdot v) \cdot 4\sqrt{2} d\eta \\ &= 2\sqrt{2} \sigma h \int_{-1}^1 (-u + v) d\eta \\ &= 2\sqrt{2} \sigma h \int_{-1}^1 (b_{ij} - a_{ij}) \eta^i d\eta \quad ? \end{aligned}$$

Lagrange Multiplier:

In mathematical optimization, the method of Lagrange multipliers is a strategy for finding the local maxima and minima of a function subject to equality constraints (i.e., subject to the condition that one or more equations have to be satisfied exactly by the chosen values of the variables).

Method : in order to find the max or min of a function $f(x)$ subject to the equality constraints $g(x) = 0$

$$L(x, \lambda) = f(x) + \lambda g(x)$$

and find the stationary points of L considered as a function of x and the Lagrange multiplier λ .

$$L(x, y, \lambda) = f(x, y) + \lambda \underbrace{g(x, y)}_{\text{constraint } g(x, y) = 0}$$

Gradient :

$$\nabla_{x,y,\lambda} L(x, y, \lambda) = \left(\frac{\partial L}{\partial x}, \frac{\partial L}{\partial y}, \frac{\partial L}{\partial \lambda} \right) = 0$$

In this case :

$$f(a_{ij}, b_{ij}) = U(a_{ij}, b_{ij}) - T(a_{ij}, b_{ij})$$

$$g(a_{ij}, b_{ij}) = R_{m \times 1} \quad m = ? : \text{dependents on \# of constraints}$$

$\lambda = 1 \times m$ Matrix

$$\frac{\partial f}{\partial a_{ij}} = \lambda \frac{\partial g}{\partial a_{ij}} \quad \frac{\partial f}{\partial b_{ij}} = \lambda \frac{\partial g}{\partial b_{ij}} \quad g(a_{ij}, b_{ij}) = 0$$

To be more general :

parameter a has a dimension of $i \times j$

parameter b has a dimension of $p \times q$

So, a_{ij} has $(i+1)(j+1)$ elements

b_{pq} has $(p+1)(q+1)$ elements

λ_m has m elements

$\uparrow m$ depends on how many edges
have been constrained

Therefore, totally $[(i+1)(j+1) + (p+1)(q+1) + m]$
unknowns to be solved.

Equation Group #1: $\frac{\partial f}{\partial a_{ij}} = \lambda_m \frac{\partial g}{\partial a_{ij}}$

has $(i+1)(j+1)$ equations

Equation Group #2: $\frac{\partial f}{\partial b_{pq}} = \lambda_m \frac{\partial g}{\partial b_{pq}}$

has $(p+1)(q+1)$ equations

Equation Group #3: $g(a_{ij}, b_{pq}) = 0$

has m equations

Therefore, totally $[(i+1)(j+1) + (p+1)(q+1) + m]$
equations to use.

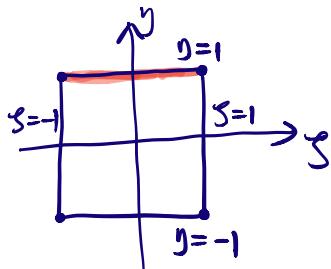
Should be solvable !!!

Constraints R

First constrain one edge, set displacement (u and v) on the side $j=1$ to zero:

$$u(s, 1) = \sum_i \sum_j a_{ij} s^i (1)^j = \sum_i \sum_j a_{ij} s^i = 0$$

$$v(s, 1) = \sum_i \sum_j b_{ij} s^i (1)^j = \sum_i \sum_j b_{ij} s^i = 0$$



$$u(s, 1) = \sum_{i=0}^{\infty} \sum_{j=0}^{\infty} a_{ij} s^i = 0$$

$$\stackrel{i=0}{\rightarrow} = a_{00} s^0 + a_{01} s^0 + a_{02} s^0 + a_{03} s^0 + a_{04} s^0 + \dots$$

$$\stackrel{i=1}{\rightarrow} + a_{10} s^1 + a_{11} s^1 + a_{12} s^1 + a_{13} s^1 + a_{14} s^1 + \dots$$

$$\stackrel{i=2}{\rightarrow} + a_{20} s^2 + a_{21} s^2 + a_{22} s^2 + a_{23} s^2 + a_{24} s^2 + \dots$$

$$\stackrel{i=i}{\rightarrow} + \dots + a_{i0} s^i + a_{i1} s^i + a_{i2} s^i + a_{i3} s^i + a_{i4} s^i + \dots$$

$$= [a_{00} + a_{01} + a_{02} + a_{03} + a_{04} + \dots] s^0$$

$$+ [a_{10} + a_{11} + a_{12} + a_{13} + a_{14} + \dots] s^1$$

$$+ [a_{20} + a_{21} + a_{22} + a_{23} + a_{24} + \dots] s^2$$

+

$$+ [a_{i0} + a_{i1} + a_{i2} + a_{i3} + a_{i4} + \dots] s^i$$

\Rightarrow Constrain Equations:

$$R \left\{ \begin{array}{l} a_{00} + a_{01} + a_{02} + a_{03} + a_{04} + \dots + a_{0j} = 0 \\ a_{10} + a_{11} + a_{12} + a_{13} + a_{14} + \dots + a_{1j} = 0 \\ a_{20} + a_{21} + a_{22} + a_{23} + a_{24} + \dots + a_{2j} = 0 \\ \dots \\ a_{i0} + a_{i1} + a_{i2} + a_{i3} + a_{i4} + \dots + a_{ij} = 0 \end{array} \right\} (i+1) \text{ equations}$$

$\Rightarrow (i+1)$ constraint equations for a_{ij} :

$$a_{i0} + a_{i1} + a_{i2} + \dots + a_{ij} = 0$$

(i+1) constraint equations for b_{ij} :

$$b_{i0} + b_{i1} + b_{i2} + \dots + b_{ij} = 0$$

Boundary conditions for plane stress

- u and v are in-plane deformation
no relations with bending moment
and reaction load in plane stress.
- u and v helps with transverse displacements
- Four edges
 - $\xi=1, \xi=-1, \eta=-1$ and $\eta=1$
- In plane stress situation, u and v are transverse displacements not related to moment and reaction load
Therefore, there are 2 cases here:

constrained

$$u(\xi, \eta) = 0$$

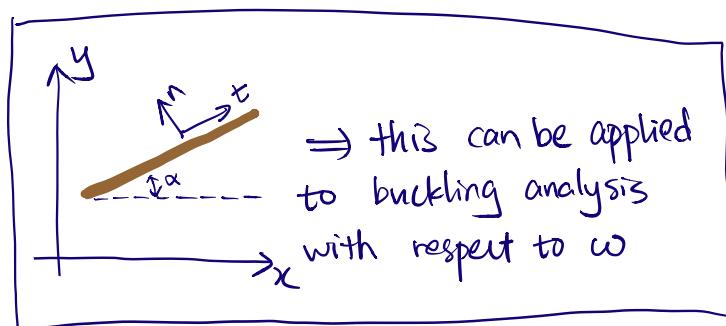
$$v(\xi, \eta) = 0$$

v.s.

not constrained

$$u(\xi, \eta) = \text{free}$$

$$v(\xi, \eta) = \text{free}$$



Apply Minimum Potential Energy

$$\Pi = U - T$$

$$d\Pi = \left(\frac{\partial U}{\partial a_{ij}} - \frac{\partial T}{\partial a_{ij}} \right) da_{ij} + \left(\frac{\partial U}{\partial b_{ij}} - \frac{\partial T}{\partial b_{ij}} \right) db_{ij} = 0$$

$$[K] \sim \frac{\partial U}{\partial a_{ij}} \text{ & } \frac{\partial U}{\partial b_{ij}} \leftrightarrow \text{Minimization of } U$$

$$[F] \sim \frac{\partial T}{\partial a_{ij}} \text{ & } \frac{\partial T}{\partial b_{ij}} \leftrightarrow \text{Minimization of } T$$

$$\{x\} \sim a_{ij} \text{ & } b_{ij}$$

$$\bar{\Pi} = U - T + \lambda_i R_i$$

$$d\bar{\Pi} = \left(\frac{\partial U}{\partial a_{ij}} - \frac{\partial T}{\partial a_{ij}} + \frac{\partial \lambda_i R_i}{\partial a_{ij}} \right) da_{ij} + \left(\frac{\partial U}{\partial b_{ij}} - \frac{\partial T}{\partial b_{ij}} + \frac{\partial \lambda_i R_i}{\partial b_{ij}} \right) db_{ij} \\ + \left(\frac{\partial U}{\partial \lambda_i} - \frac{\partial T}{\partial \lambda_i} + \frac{\partial \lambda_i R_i}{\partial \lambda_i} \right) d\lambda_i = 0$$

$$\begin{bmatrix} K & TR^T \\ TR & 0_{m \times 1} \end{bmatrix} \begin{Bmatrix} x \\ \lambda \end{Bmatrix} = \begin{Bmatrix} F \\ 0 \end{Bmatrix} \Rightarrow \text{eigenvalue problem}$$

→ coefficient of u and v : a_{ij} & b_{ij}

→ stress & strain throughout the plate.

Buckling (Bending)

Displacement Function : out-of-plane displacement w

$$w = \sum_i \sum_j c_{ij} \sin j$$

Strain Energy ✓

$$U = \frac{D}{2} \int_A \left\{ \left(\frac{\partial^2 w}{\partial x^2} + \frac{\partial^2 w}{\partial y^2} \right)^2 - 2(1-\nu) \left[\frac{\partial^2 w}{\partial x^2} \frac{\partial^2 w}{\partial y^2} - \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \right] \right\} dA$$

$$D = \frac{Eh^3}{12(1-\nu^2)}$$

\Downarrow in (x, y) domain

$$U = \frac{D}{2} \int_{-1}^1 \int_{-1}^1 \left\{ \left(\frac{\partial^2 w}{\partial x^2} + \frac{\partial^2 w}{\partial y^2} \right)^2 - 2(1-\nu) \left[\frac{\partial^2 w}{\partial x^2} \frac{\partial^2 w}{\partial y^2} - \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \right] \right\} |J| d\zeta d\eta$$

\Downarrow in (ξ, η) domain

$$|J| = \frac{\partial x}{\partial \zeta} \cdot \frac{\partial y}{\partial \eta} - \frac{\partial x}{\partial \eta} \cdot \frac{\partial y}{\partial \zeta}$$

$$\frac{\partial w}{\partial x} = \frac{\partial w}{\partial \zeta} \frac{\partial \zeta}{\partial x} + \frac{\partial w}{\partial \eta} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial w}{\partial y} = \frac{\partial w}{\partial \zeta} \frac{\partial \zeta}{\partial y} + \frac{\partial w}{\partial \eta} \frac{\partial \eta}{\partial y}$$

$$\frac{\partial^2 w}{\partial x^2} = \frac{\partial^2 w}{\partial \zeta^2} \left(\frac{\partial \zeta}{\partial x} \right)^2 + \frac{\partial^2 w}{\partial \eta^2} \left(\frac{\partial \eta}{\partial x} \right)^2 + \frac{\partial w}{\partial \eta} \frac{\partial^2 \zeta}{\partial x^2} + \frac{\partial w}{\partial \zeta} \frac{\partial^2 \eta}{\partial x^2} + 2 \frac{\partial^2 w}{\partial \zeta \partial \eta} \frac{\partial \zeta}{\partial x} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial^2 w}{\partial y^2} = \frac{\partial^2 w}{\partial \zeta^2} \left(\frac{\partial \zeta}{\partial y} \right)^2 + \frac{\partial^2 w}{\partial \eta^2} \left(\frac{\partial \eta}{\partial y} \right)^2 + \frac{\partial w}{\partial \eta} \frac{\partial^2 \zeta}{\partial y^2} + \frac{\partial w}{\partial \zeta} \frac{\partial^2 \eta}{\partial y^2} + 2 \frac{\partial^2 w}{\partial \zeta \partial \eta} \frac{\partial \zeta}{\partial y} \frac{\partial \eta}{\partial y}$$

$$\begin{aligned} \frac{\partial^2 w}{\partial x \partial y} &= \frac{\partial^2 w}{\partial \zeta \partial \eta} \left(\frac{\partial \zeta}{\partial x} \frac{\partial \eta}{\partial y} + \frac{\partial \zeta}{\partial y} \frac{\partial \eta}{\partial x} \right) + \frac{\partial^2 w}{\partial \zeta^2} \frac{\partial \zeta}{\partial x} \frac{\partial \eta}{\partial y} \\ &\quad + \frac{\partial^2 w}{\partial \eta^2} \frac{\partial \eta}{\partial x} \frac{\partial \zeta}{\partial y} + \frac{\partial w}{\partial \zeta} \frac{\partial^2 \eta}{\partial x \partial y} + \frac{\partial w}{\partial \eta} \frac{\partial^2 \zeta}{\partial x \partial y} \end{aligned}$$

Work of External Loads



$$T = \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left[N_x \left(\frac{\partial w}{\partial \xi} \right)^2 + N_y \left(\frac{\partial w}{\partial \eta} \right)^2 + 2 N_{xy} \frac{\partial w}{\partial \xi} \frac{\partial w}{\partial \eta} \right] \cdot |J(\xi, \eta)| d\xi d\eta$$

— T is evaluated in (ξ, η) domain using Gaussian integration, with N_x , N_y and N_{xy} defined at each Gaussian point.

$$\begin{cases} N_x = \sigma_{xx} h \\ N_y = \sigma_{yy} h \\ N_{xy} = \sigma_{xy} h \end{cases} \quad \leftarrow \text{with the stresses at the Gauss point yielding } N_x, N_y, N_{xy}.$$

$$[\sigma] = [G] \{ \varepsilon \}$$

$$[G] = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{(1-\nu)}{2} \end{bmatrix}$$

$$\{ \varepsilon \} = \begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \left\{ \frac{\partial u}{\partial x}, \frac{\partial v}{\partial y}, \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right\}^T$$

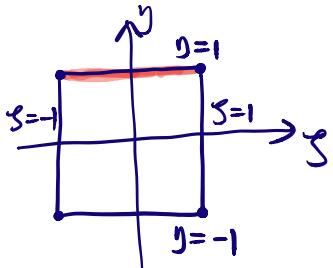
$$[\sigma] = \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} \quad \leftarrow \text{Results from plane stress}$$

$$|J| = \frac{\partial x}{\partial \xi} \cdot \frac{\partial y}{\partial \eta} - \frac{\partial x}{\partial \eta} \cdot \frac{\partial y}{\partial \xi}$$

$$\frac{\partial w}{\partial x} = \frac{\partial w}{\partial \xi} \frac{\partial \xi}{\partial x} + \frac{\partial w}{\partial \eta} \frac{\partial \eta}{\partial x}$$

$$\frac{\partial w}{\partial y} = \frac{\partial w}{\partial \xi} \frac{\partial \xi}{\partial y} + \frac{\partial w}{\partial \eta} \frac{\partial \eta}{\partial y}$$

Buckling Constraints



$$w = \sum_i \sum_j c_{ij} s^i y^j$$

Analysis will be similar to plate out-of-plane bending, with special case of a square

Therefore, ① along $y=1$ or $y=-1$:

$$w_y = w|_{y=\pm 1}$$

$$M_y = \left(\frac{\partial^2 w}{\partial y^2} + D \frac{\partial^2 w}{\partial s^2} \right) \Big|_{y=\pm 1}$$

$$R_y = \frac{\partial}{\partial y} \left(\frac{\partial^2 w}{\partial y^2} + (2-D) \frac{\partial^2 w}{\partial s^2} \right) \Big|_{y=\pm 1}$$

$$\text{Slope}_y = \frac{\partial w}{\partial y} \Big|_{y=\pm 1}$$

② Along $s=1$ or $s=-1$

$$w_s = w|_{s=\pm 1}$$

$$M_s = \left(\frac{\partial^2 w}{\partial s^2} + D \frac{\partial^2 w}{\partial y^2} \right) \Big|_{s=\pm 1}$$

$$R_s = \frac{\partial}{\partial s} \left(\frac{\partial^2 w}{\partial s^2} + (2-D) \frac{\partial^2 w}{\partial y^2} \right) \Big|_{s=\pm 1}$$

$$\text{Slope}_s = \frac{\partial w}{\partial s} \Big|_{s=\pm 1}$$

Apply Minimum Potential Energy

$$\bar{\Pi} = U - T + \lambda_i R_i$$

Minimization of $\bar{\Pi}$ yields the critical loads :

$$\frac{\partial \bar{\Pi}}{\partial c_{ij}} = 0 \quad \text{and} \quad \frac{\partial \bar{\Pi}}{\partial \lambda_i} = 0$$

This gives the eigenvalue problem :

$$\left(\begin{bmatrix} \frac{\partial U}{\partial c_{ij}} & R_i^T \\ R_i & 0 \end{bmatrix} - P \begin{bmatrix} \frac{\partial T}{\partial c_{ij}} & 0 \\ 0 & 0 \end{bmatrix} \right) \begin{Bmatrix} c_{ij} \\ \lambda_i \end{Bmatrix} = 0$$

P : critical load factor